



Behaviour of a Kirshhoff rod loaded by a pure moment

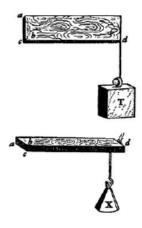
Marwan Hariz

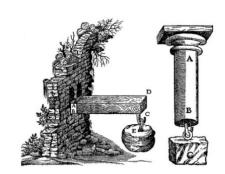
Joint work with Loic Le Marrec and Jean Lerbet

Rencontre du GDR GDM à La Rochelle 7-9 Juillet 2021

- Problem statement
- Solution of the problem
- 3 Further analysis:rod shapes
- 4 Conclusion

Beam:structure element that resists loads





L. Da Vinci 1493 G. Galilei 1638

Rod/Beam theories

Kirshhoff Rod





Kinematical constraint Constrained load Moments

G. Kirchhoff 1859

Beam

Euler-Bernoulli

Rod/Beam theories

Kirshhoff Rod

Problem statement 000000









Kinematical constraint Constrained load Moments

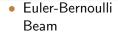


Kinematical constaint Any type of load Moments

Rod/Beam theories

Kirshhoff Rod









 Timoshenko beam





Kinematical constraint Constrained load Moments



Kinematical constaint
Any type of load
Moments

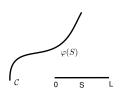


Shear effect
Any type of load
Moments

Problem statement 000000

Cosserat beam model

- Material curve C.
- S curvilinear coordinate of C.
- $\varphi(S)$ placement of C.

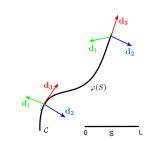


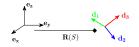
Kirshhoff rod model

Problem statement 000000

Cosserat beam model

- Material curve C.
- S curvilinear coordinate of C.
- $\varphi(S)$ placement of C.
- Moving director frame $\{d_i\} := (d_1, d_2, d_3)$ d_3 tangent to the centerline.



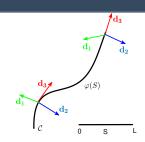


Kirshhoff rod model

Problem statement

Cosserat beam model

- Material curve C.
- S curvilinear coordinate of C.
- $\varphi(S)$ placement of C.
- Moving director frame
 {d_i} := (d₁, d₂, d₃)
 d₃ tangent to the centerline.



With

$$\frac{d\mathbf{d_i}}{dS} = \kappa \times \mathbf{d_i}$$



Generalised curvatures

$$\boldsymbol{\kappa} = \kappa_1 \boldsymbol{d}_1 + \kappa_2 \boldsymbol{d}_2 + \kappa_3 \boldsymbol{d}_3$$

Basic assumption

Internal forces and moment

$$N = N_1 d_1 + N_2 d_2 + N_3 d_3$$

 $M = M_1 d_1 + M_2 d_2 + M_3 d_3$

With

 N_1, N_2 shear forces N_3 normal force M_1, M_2 bending moments M_3 torsion

Basic assumption

Internal forces and moment

$$N = N_1 d_1 + N_2 d_2 + N_3 d_3$$

 $M = M_1 d_1 + M_2 d_2 + M_3 d_3$

With

 N_1, N_2 shear forces N_3 normal force M_1, M_2 bending moments M_3 torsion

$$\frac{d\mathbf{N}}{dS} = 0$$

$$\frac{d\mathbf{M}}{dS} + \mathbf{d}_3 \times \mathbf{N} = 0$$

Internal forces and moment

$$N = N_1 d_1 + N_2 d_2 + N_3 d_3$$

 $M = M_1 d_1 + M_2 d_2 + M_3 d_3$

$\frac{d\mathbf{N}}{dS} = 0 \Longrightarrow \mathbf{N}(S) = \mathbf{N}$

With

 N_1, N_2 shear forces N_3 normal force M_1, M_2 bending moments M_3 torsion

$$\frac{d\mathbf{N}}{dS} = 0$$

$$\frac{d\mathbf{M}}{dS} + \mathbf{d}_3 \times \mathbf{N} = 0$$

Internal forces and moment

$$N = N_1 d_1 + N_2 d_2 + N_3 d_3$$

 $M = M_1 d_1 + M_2 d_2 + M_3 d_3$

With

000000

 N_1, N_2 shear forces N_3 normal force M_1, M_2 bending moments M_3 torsion

$$\frac{d\mathbf{N}}{dS} = 0 \Longrightarrow \mathbf{N}(S) = \mathbf{N}$$

No external forces

$$N = 0$$

$$\frac{d\mathbf{N}}{dS} = 0$$

$$\frac{d\mathbf{M}}{dS} + \mathbf{d}_3 \times \mathbf{N} = 0$$

Basic assumption

Internal forces and moment

$$N = N_1 d_1 + N_2 d_2 + N_3 d_3$$

 $M = M_1 d_1 + M_2 d_2 + M_3 d_3$

With

000000

 N_1, N_2 shear forces N_3 normal force M_1, M_2 bending moments M_3 torsion

$$\frac{d\mathbf{N}}{dS} = 0 \Longrightarrow \mathbf{N}(S) = \mathbf{N}$$

No external forces

$$N = 0$$

$$\frac{d\mathbf{M}}{dS}=0$$

$$\frac{d\mathbf{N}}{dS} = 0$$

$$\frac{d\mathbf{M}}{dS} + \mathbf{d}_3 \times \mathbf{N} = 0$$

Basic assumption

Internal forces and moment

$$\mathbf{N} = N_1 \mathbf{d}_1 + N_2 \mathbf{d}_2 + N_3 \mathbf{d}_3$$

 $\mathbf{M} = M_1 \mathbf{d}_1 + M_2 \mathbf{d}_2 + M_3 \mathbf{d}_3$

With

 N_1, N_2 shear forces N_3 normal force M_1, M_2 bending moments M_3 torsion

Static equilibrium

$$\frac{d\mathbf{N}}{dS} = 0$$

$$\frac{d\mathbf{M}}{dS} + \mathbf{d}_3 \times \mathbf{N} = 0$$

$\frac{d\mathbf{N}}{dS} = 0 \Longrightarrow \mathbf{N}(S) = \mathbf{N}$

No external forces

$$N = 0$$

$$\frac{d\mathbf{M}}{dS} = 0$$

Projecting along d_i

$$\frac{dM_1}{dS} + M_3\kappa_2 - M_2\kappa_3 = 0$$

$$\frac{dM_2}{dS} + M_1\kappa_3 - M_3\kappa_1 = 0$$

$$\frac{dM_3}{dS} + M_2\kappa_1 - M_1\kappa_2 = 0$$

$$\frac{dM_3}{dS} + M_2\kappa_1 - M_1\kappa_2 = 0$$

Constitutive laws

000000

St. Venant Kirshhoff energy per unit length

$$\psi = \frac{1}{2} \left(EI_1 \kappa_1^2 + EI_2 \kappa_2^2 + GI_3 \kappa_3^2 \right)$$

Linear constitutive laws

$$M_1 = EI_1 \kappa_1$$
 $M_2 = EI_2 \kappa_2$ $M_3 = GI_3 \kappa_3$

Equilibrium

$$EI_{1}\frac{d\kappa_{1}}{dS} + (GI_{3} - EI_{2})\kappa_{2}\kappa_{3} = 0$$

$$EI_{2}\frac{d\kappa_{2}}{dS} + (EI_{1} - GI_{3})\kappa_{1}\kappa_{3} = 0$$

$$GI_{3}\frac{d\kappa_{3}}{dS} + E(I_{2} - I_{1})\kappa_{1}\kappa_{2} = 0$$

$$GI_3 \frac{d\kappa_3}{dS} + E(I_2 - I_1)\kappa_1\kappa_2 = 0$$

Dimensionless procedure

Dimensionless parameters

$$\varrho = \sqrt{\frac{l_1 + l_2}{A}} \qquad s = \frac{s}{\varrho} \qquad \ell = \frac{L}{\varrho}$$

$$=\frac{S}{\rho}$$
 $\ell=\frac{L}{\rho}$

$$g = \frac{E}{G}$$
 $e = \frac{I_1}{I_2}$

with

 $I_1 < I_2 < I_3$

 $0 < e \le 1$, $2 \lesssim g \lesssim 3$

Kinematical variables

$$\varphi_i(s) = \frac{1}{\varrho} \underline{\varphi}_i(S)$$

$$\kappa_i(s) = \varrho \underline{\kappa}_i(S)$$

$$2 \lesssim g \lesssim 3$$

Dimensionless variables

Dimensionless parameters

$$g = \frac{E}{G}$$

$$e=\frac{I_1}{I_2}$$

$$2 \lesssim g \lesssim 3$$
 $0 < e \leq 1$

Bending stiffness

$$r_1 := \frac{eg}{1+e}$$

$$r_2:=\frac{g}{1+e}$$

Moments

$$M_1 = r_1 \kappa_1$$
 $M_2 = r_2 \kappa_1$ $M_3 = \kappa_3$

Energy per unit length

$$\Psi(s) = \frac{1}{2} \left(r_1 \, \kappa_1^2 + r_2 \, \kappa_2^2 + \kappa_3^2 \right)$$

Curvature formulation

$$r_1 \kappa_1'(s) - (r_2 - 1) \kappa_2(s) \kappa_3(s) = 0$$

$$r_2 \kappa_2'(s) + (r_1 - 1) \kappa_1(s) \kappa_3(s) = 0$$

$$\kappa_3'(s) + (r_2 - r_1) \kappa_1(s) \kappa_2(s) = 0$$

Non linear first order Similar to Euler's rotation

D.F. Lawden 2013

Geometrical regimes (material + cross section shapes)

Dimensionless parameters

$$g = \frac{E}{G}$$

$$g = \frac{E}{G} \qquad \qquad e = \frac{I_1}{I_2}$$

$$0 < e \le 1$$
 $2 \lesssim g \lesssim 3$

Bending stiffness

$$r_1 := \frac{eg}{1+e}$$

$$r_2:=\frac{g}{1+e}$$

Cross section properties

$$r_1 = r_2$$
 i.e. $e = 1$

$$e = 1$$

$$1 < r_1$$
 i.e. $e > \frac{1}{g-1}$

$$r_1 < 1$$
 i.e. $e < \frac{1}{g-1}$

Thin cross-section (ribbon-like rod)

Homogeneous solutions

Equilibrium

$$r_1 \kappa'_1(s) - (r_2 - 1) \kappa_2(s) \kappa_3(s) = 0$$

 $r_2 \kappa'_2(s) + (r_1 - 1) \kappa_1(s) \kappa_3(s) = 0$
 $\kappa'_3(s) + (r_2 - r_1) \kappa_1(s) \kappa_2(s) = 0$

Bending % d_1

$$\kappa_1(s) = \kappa_{10}$$
 $\kappa_2(s) = 0$
 $\kappa_3(s) = 0$

Bending % d_2

$$\kappa_1(s) = 0$$
 $\kappa_2(s) = \kappa_{20}$
 $\kappa_3(s) = 0$

Pure torsion

$$\kappa_1(s) = 0$$
 $\kappa_2(s) = 0$
 $\kappa_3(s) = \kappa_{30}$

$$e
eq 1$$
 and $r_1
eq 1$

Trivial solutions

Equilibrium

$$r_1 \kappa'_1(s) - (r_2 - 1) \kappa_2(s) \kappa_3(s) = 0$$

$$r_2 \kappa'_2(s) + (r_1 - 1) \kappa_1(s) \kappa_3(s) = 0$$

$$\kappa'_3(s) + (r_2 - r_1) \kappa_1(s) \kappa_2(s) = 0$$

$$1 < r_1 = r_2$$
 (i.e. $e = 1$)

$$\kappa_{1}(s) = \kappa_{10} \cos\left(\frac{g-2}{g}\kappa_{30}s\right) + \kappa_{20} \sin\left(\frac{g-2}{g}\kappa_{30}s\right)$$

$$\kappa_{2}(s) = \kappa_{20} \cos\left(\frac{g-2}{g}\kappa_{30}s\right) - \kappa_{10} \sin\left(\frac{g-2}{g}\kappa_{30}s\right)$$

$$\kappa_{3}(s) = \kappa_{30}$$

$$r_{1} = r_{2} = 1$$

$$\kappa_{1}(s) = \kappa_{10}$$

$$\kappa_{2}(s) = \kappa_{20}$$

$$\kappa_{3}(s) = \kappa_{30}$$

$$r_1 = r_2 = 1$$
 $\kappa_1(s) = \kappa_{10}$
 $\kappa_2(s) = \kappa_{20}$
 $\kappa_3(s) = \kappa_{30}$

Trivial solutions

Equilibrium

$$r_1 \kappa'_1(s) - (r_2 - 1) \kappa_2(s) \kappa_3(s) = 0$$

 $r_2 \kappa'_2(s) + (r_1 - 1) \kappa_1(s) \kappa_3(s) = 0$
 $\kappa'_3(s) + (r_2 - r_1) \kappa_1(s) \kappa_2(s) = 0$

$$1 < r_1 = r_2$$
 (i.e. $e = 1$)

$$\kappa_{1}(s) = \kappa_{10} \cos\left(\frac{g-2}{g}\kappa_{30} s\right) + \kappa_{20} \sin\left(\frac{g-2}{g}\kappa_{30} s\right)$$

$$\kappa_{2}(s) = \kappa_{20} \cos\left(\frac{g-2}{g}\kappa_{30} s\right) - \kappa_{10} \sin\left(\frac{g-2}{g}\kappa_{30} s\right)$$

 $r_1 = r_2 = 1$ $\kappa_1(s) = \kappa_{10}$ $\kappa_2(s) = \kappa_{20}$ $\kappa_3(s) = \kappa_{30}$

$$r_1 = 1 < r_2$$

 $\kappa_3(s) = \kappa_{30}$

$$\kappa_1(s) = \kappa_{10} \cos \left((g-2)\kappa_{20} s \right) + \kappa_{30} \sin \left((g-2)\kappa_{20} s \right)$$
 $\kappa_2(s) = \kappa_{20}$

$$\kappa_{3}(s) = \kappa_{20}$$

 $\kappa_{3}(s) = \kappa_{30} \cos((g-2)\kappa_{20} s) - \kappa_{10} \sin((g-2)\kappa_{20} s)$

 ${\it M}$ is ${\it uniform}$ all along the rod

 $||\boldsymbol{M}|| := M$ is constant.

M is **uniform** all along the rod

$$||\boldsymbol{M}|| := M$$
 is constant.

First invariant: Moment

$$M^2 = (r_1 \kappa_1(s))^2 + (r_2 \kappa_2(s))^2 + \kappa_3(s)^2$$

M is **uniform** all along the rod

$$||\boldsymbol{M}|| := M$$
 is constant.

First invariant: Moment

$$M^2 = (r_1 \kappa_1(s))^2 + (r_2 \kappa_2(s))^2 + \kappa_3(s)^2$$

Second invariant: Energy per unit length

$$\psi = \frac{1}{2} \Big(r_1 \, \kappa_1(s)^2 + r_2 \, \kappa_2(s)^2 + \kappa_3(s)^2 \Big)$$

M is **uniform** all along the rod

$$||\boldsymbol{M}|| := M$$
 is constant.

First invariant: Moment

$$M^2 = (r_1 \kappa_1(s))^2 + (r_2 \kappa_2(s))^2 + \kappa_3(s)^2$$

Second invariant: Energy per unit length

$$\psi = \frac{1}{2} \Big(r_1 \, \kappa_1(s)^2 + r_2 \, \kappa_2(s)^2 + \kappa_3(s)^2 \Big)$$

$$\mu^2 := 2\psi$$

$$M^2 = M_1^2 + M_2^2 + M_3^2$$

$$\left[\mu^2 = rac{1}{r_1} M_1^2 + rac{1}{r_2} M_2^2 + M_3^2
ight]$$

Sphere

Ellipsoid

In (M_1, M_2, M_3) Configuration

$$M^2 = M_1^2 + M_2^2 + M_3^2$$

$$\int \mu^2 = rac{1}{r_1} M_1^2 + rac{1}{r_2} M_2^2 + M_3^2$$

Sphere

Ellipsoid

In (M_1, M_2, M_3) Configuration

Solutions Surfaces intersections

Existence condition

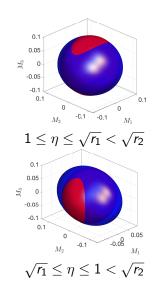
$$\min_{i=1,2} (r_i,1) \leq \eta := \frac{M}{\mu} \leq \max_{i=1,2} (r_i,1)$$

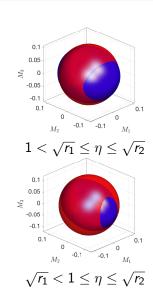
Geometrical representation with fixed energy μ

Thick

$$\eta = \frac{M}{\mu}$$

Thin





$$r_1 \kappa_1'(s) - (r_2 - 1) \kappa_2(s) \kappa_3(s) = 0$$

$$r_2 \kappa_2'(s) + (r_1 - 1) \kappa_1(s) \kappa_3(s) = 0$$

$$\kappa_3'(s) + (r_2 - r_1) \kappa_1(s) \kappa_2(s) = 0$$

$$r_1 \kappa'_1(s) - (r_2 - 1) \kappa_2(s) \kappa_3(s) = 0$$

 $r_2 \kappa'_2(s) + (r_1 - 1) \kappa_1(s) \kappa_3(s) = 0$
 $\kappa'_3(s) + (r_2 - r_1) \kappa_1(s) \kappa_2(s) = 0$

For thick rod

$$r_1 \kappa'_1(s) - (r_2 - 1) \kappa_2(s) \kappa_3(s) = 0$$

 $r_2 \kappa'_2(s) + (r_1 - 1) \kappa_1(s) \kappa_3(s) = 0$
 $\kappa'_3(s) + (r_2 - r_1) \kappa_1(s) \kappa_2(s) = 0$

For thick rod

$$1 \le \eta \le \sqrt{r_1} < \sqrt{r_2}$$

$$\kappa_1(s) = \overline{\kappa}_1 \operatorname{sn}(\lambda(s+s_0) \mid m)$$

$$\kappa_2(s) = \overline{\kappa}_2 \operatorname{cn}(\lambda(s+s_0) \mid m)$$

$$\kappa_3(s) = \pm \overline{\kappa}_3 \operatorname{dn}(\lambda(s+s_0) \mid m)$$

$$1 < \sqrt{r_1} \le \eta \le \sqrt{r_2}$$

$$\kappa_1(s) = \overline{\kappa}_1 \operatorname{sn}(\lambda(s+s_0) \mid m)$$
 $\kappa_2(s) = \pm \overline{\kappa}_2 \operatorname{dn}(\lambda(s+s_0) \mid m)$
 $\kappa_3(s) = \overline{\kappa}_3 \operatorname{cn}(\lambda(s+s_0) \mid m)$

$$r_1 \kappa'_1(s) - (r_2 - 1) \kappa_2(s) \kappa_3(s) = 0$$

 $r_2 \kappa'_2(s) + (r_1 - 1) \kappa_1(s) \kappa_3(s) = 0$
 $\kappa'_3(s) + (r_2 - r_1) \kappa_1(s) \kappa_2(s) = 0$

For thick rod

$$1 \le \eta \le \sqrt{r_1} < \sqrt{r_2}$$

$$\kappa_1(s) = \overline{\kappa}_1 \operatorname{sn}(\lambda(s+s_0) \mid m)$$

$$\kappa_2(s) = \overline{\kappa}_2 \operatorname{cn}(\lambda(s+s_0) \mid m)$$

$$\kappa_2(s) = \overline{\kappa}_2 \operatorname{cn}(\lambda(s+s_0) \mid m)$$

$$\kappa_3(s) = \pm \overline{\kappa}_3 \operatorname{dn}(\lambda(s+s_0) \mid m)$$

$$1 < \sqrt{r_1} \le \eta \le \sqrt{r_2}$$

$$egin{array}{lll} \kappa_1(s) &=& \overline{\kappa}_1 \operatorname{sn}(\lambda(s+s_0) \mid m) \ \kappa_2(s) &=& \pm \overline{\kappa}_2 \operatorname{dn}(\lambda(s+s_0) \mid m) \ \kappa_3(s) &=& \overline{\kappa}_3 \operatorname{cn}(\lambda(s+s_0) \mid m) \end{array}$$

$$r_1 \kappa'_1(s) - (r_2 - 1) \kappa_2(s) \kappa_3(s) = 0$$

$$r_2 \kappa'_2(s) + (r_1 - 1) \kappa_1(s) \kappa_3(s) = 0$$

$$\kappa'_3(s) + (r_2 - r_1) \kappa_1(s) \kappa_2(s) = 0$$

For thick rod

$$1 \le \eta \le \sqrt{r_1} < \sqrt{r_2}$$

$$\kappa_1(s) = \overline{\kappa}_1 \operatorname{sn}(\lambda(s+s_0) \mid m)$$

$$\kappa_2(s) = \overline{\kappa}_2 \operatorname{cn}(\lambda(s+s_0) \mid m)$$

$$\kappa_3(s) = \pm \overline{\kappa}_3 \operatorname{dn}(\lambda(s+s_0) \mid m)$$

For thin rod

$$\sqrt{r_1} \le \eta \le 1 < \sqrt{r_2}$$

$$\kappa_1(s) = \pm \overline{\kappa}_1 \operatorname{dn}(\lambda(s+s_0) \mid m)$$

$$\kappa_2(s) = \overline{\kappa}_2 \operatorname{cn}(\lambda(s+s_0) \mid m)$$

$$\kappa_3(s) = \overline{\kappa}_3 \operatorname{sn}(\lambda(s+s_0) \mid m)$$

$$1 < \sqrt{r_1} \le \eta \le \sqrt{r_2}$$

$$\kappa_1(s) = \overline{\kappa}_1 \operatorname{sn}(\lambda(s+s_0) \mid m)$$
 $\kappa_2(s) = \pm \overline{\kappa}_2 \operatorname{dn}(\lambda(s+s_0) \mid m)$
 $\kappa_3(s) = \overline{\kappa}_3 \operatorname{cn}(\lambda(s+s_0) \mid m)$

$$\sqrt{r_1} < 1 \le \eta \le \sqrt{r_2}$$

$$\kappa_1(s) = \overline{\kappa}_1 \operatorname{cn}(\lambda(s+s_0) \mid m)$$
 $\kappa_2(s) = \pm \overline{\kappa}_2 \operatorname{dn}(\lambda(s+s_0) \mid m)$
 $\kappa_3(s) = \overline{\kappa}_3 \operatorname{sn}(\lambda(s+s_0) \mid m)$

$$r_1 \kappa'_1(s) - (r_2 - 1) \kappa_2(s) \kappa_3(s) = 0$$

 $r_2 \kappa'_2(s) + (r_1 - 1) \kappa_1(s) \kappa_3(s) = 0$
 $\kappa'_3(s) + (r_2 - r_1) \kappa_1(s) \kappa_2(s) = 0$

For thick rod

$$1 \le \eta \le \sqrt{r_1} < \sqrt{r_2}$$

$$\kappa_1(s) = \overline{\kappa}_1 \operatorname{sn}(\lambda(s+s_0) \mid m)
\kappa_2(s) = \overline{\kappa}_2 \operatorname{cn}(\lambda(s+s_0) \mid m)
\kappa_3(s) = \pm \overline{\kappa}_3 \operatorname{dn}(\lambda(s+s_0) \mid m)$$

$1 < \sqrt{r_1} \le \eta \le \sqrt{r_2}$

$$\kappa_1(s) = \overline{\kappa}_1 \operatorname{sn}(\lambda(s+s_0) \mid m)$$
 $\kappa_2(s) = \pm \overline{\kappa}_2 \operatorname{dn}(\lambda(s+s_0) \mid m)$
 $\kappa_3(s) = \overline{\kappa}_3 \operatorname{cn}(\lambda(s+s_0) \mid m)$

For thin rod

$$egin{array}{lll} \sqrt{r_1} \leq \eta \leq 1 < \sqrt{r_2} \ \kappa_1(s) &=& \pm \overline{\kappa}_1 \operatorname{dn}(\lambda(s+s_0) \mid m) \end{array}$$

$$\kappa_2(s) = \overline{\kappa}_2 \operatorname{cn}(\lambda(s+s_0) \mid m)$$

$$\kappa_3(s) = \overline{\kappa}_3 \operatorname{sn}(\lambda(s+s_0) \mid m)$$

$$\sqrt{r_1} < 1 \le \eta \le \sqrt{r_2}$$

$$\kappa_{1}(s) = \overline{\kappa}_{1} \operatorname{cn}(\lambda(s+s_{0}) \mid m)
\kappa_{2}(s) = \pm \overline{\kappa}_{2} \operatorname{dn}(\lambda(s+s_{0}) \mid m)
\kappa_{3}(s) = \overline{\kappa}_{3} \operatorname{sn}(\lambda(s+s_{0}) \mid m)$$

 $\overline{\kappa}_i, \lambda, m$ controlled by

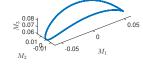
Structure r_1, r_2 Load parameters μ, η

Further analysis:rod shapes

•0000000

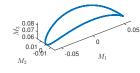
Curvature analysis w.r.t η (fixed μ)





 η fixed

Thick



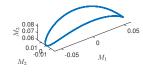


 η fixed

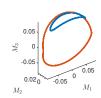
increasing η

•0000000

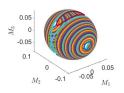




 η fixed

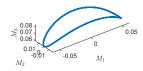


increasing η

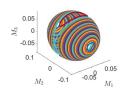


various regime





0.05 M_3 0 -0.05 0.05 0 0.02 -0.05 M_1 M_2



 η fixed

increasing η

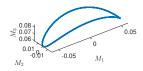
various regime

Thin

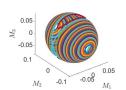


 η fixed





0.05 M_3 0 -0.05 0.05 0 0.020 -0.05 M_1 M_2



 η fixed

increasing η

various regime

Thin

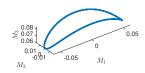


 η fixed

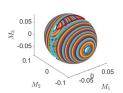


increasing η





0.05 M_3 0 -0.05 0.05 0 0.020 -0.05 M_1 M_2

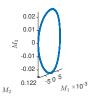


η fixed

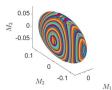
increasing η

various regime

Thin





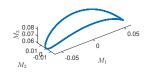


 η fixed

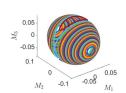
increasing η

various regime







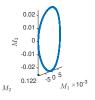


 η fixed

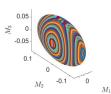
increasing η

various regime

Thin







 η fixed

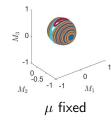
increasing η

various regime

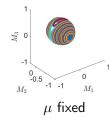
 η controls the type of the solutions

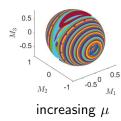
0000000

Thick



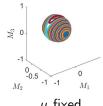
Thick



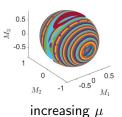


0000000

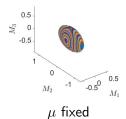
Thick



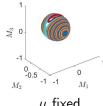
 μ fixed



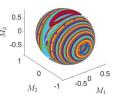
Thin



Thick

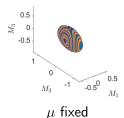


 μ fixed



increasing μ

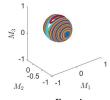
Thin



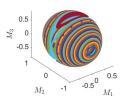
0.5 -0.5 M_2 M_1

increasing μ

Thick

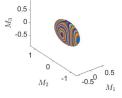


 μ fixed

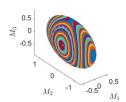


increasing μ

Thin



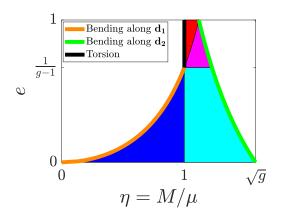
 μ fixed



increasing μ

 μ scaling parameter: size of the solution

Homogeneous solutions



$$\kappa_1(s) = \frac{\mu}{\sqrt{r_1}}$$

$$\kappa_2(s) = 0$$

$$\kappa_3(s) = 0$$

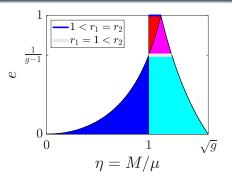
$$\kappa_1(s) = 0$$

$$\kappa_2(s) = \frac{\mu}{\sqrt{r_2}}$$

$$\kappa_3(s) = 0$$

$$\kappa_1(s) = 0$$
 $\kappa_2(s) = 0$
 $\kappa_3(s) = \mu$

Trigonometric solutions

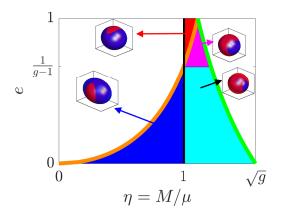


$$\kappa_1(s) = \kappa_{10} \cos\left(\frac{g-2}{g}\kappa_{30}s\right) + \kappa_{20} \sin\left(\frac{g-2}{g}\kappa_{30}s\right)$$

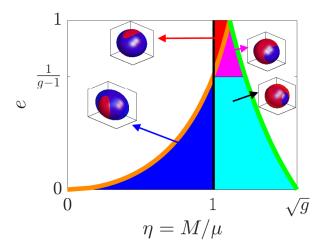
$$\kappa_2(s) = \kappa_{20} \cos\left(\frac{g-2}{g}\kappa_{30}s\right) - \kappa_{10} \sin\left(\frac{g-2}{g}\kappa_{30}s\right) \qquad \kappa_3(s) = \kappa_{30}$$

$$\kappa_1(s) = \kappa_{10} \cos((g-2)\kappa_{20} s) + \kappa_{30} \sin((g-2)\kappa_{20} s) \qquad \kappa_2(s) = \kappa_{20}
\kappa_3(s) = \kappa_{30} \cos((g-2)\kappa_{20} s) - \kappa_{10} \sin((g-2)\kappa_{20} s)$$

Jacobian solutions



Jacobian solutions



Further analysis of rod shapes gives a better understanding for the role of η and e

Further analysis:rod shapes

00000000

$$oldsymbol{arphi}' = oldsymbol{d}_3 \qquad \qquad oldsymbol{d}_i' = \kappa imes oldsymbol{d}_i$$

$$\boldsymbol{arphi'} = \boldsymbol{d}_3$$

$$d_i' = \kappa imes d_i$$

Deformed placement

$$\begin{aligned} \varphi_1' - \varphi_2 \kappa_3 + \varphi_3 \kappa_2 &= 0 \\ \varphi_2' - \varphi_3 \kappa_1 + \varphi_1 \kappa_3 &= 0 \\ \varphi_3' - \varphi_1 \kappa_2 + \varphi_2 \kappa_1 &= 1 \end{aligned}$$

Directors

$$\mathbf{d_1}' = \kappa_3 \mathbf{d_2} - \kappa_2 \mathbf{d_3}$$

$$\mathbf{d_2}' = \kappa_1 \mathbf{d_3} - \kappa_3 \mathbf{d_1}$$

$$\mathbf{d_3}' = \kappa_2 \mathbf{d_1} - \kappa_1 \mathbf{d_2}$$

Cartesian placement

$$\varphi_{\mathsf{x}} = \boldsymbol{\varphi} \cdot \boldsymbol{e}_{\mathsf{x}}
\varphi_{\mathsf{y}} = \boldsymbol{\varphi} \cdot \boldsymbol{e}_{\mathsf{y}}$$

$$\varphi_{\mathsf{z}} = \boldsymbol{\varphi} \cdot \boldsymbol{e}_{\mathsf{z}}$$

$$\varphi' = d_3$$

$$d_i' = \kappa imes d_i$$

Deformed placement

$$\varphi_1' - \varphi_2 \kappa_3 + \varphi_3 \kappa_2 = 0
 \varphi_2' - \varphi_3 \kappa_1 + \varphi_1 \kappa_3 = 0
 \varphi_3' - \varphi_1 \kappa_2 + \varphi_2 \kappa_1 = 1$$

Directors

$$\mathbf{d_1}' = \kappa_3 \mathbf{d_2} - \kappa_2 \mathbf{d_3}$$
$$\mathbf{d_2}' = \kappa_1 \mathbf{d_3} - \kappa_3 \mathbf{d_1}$$
$$\mathbf{d_3}' = \kappa_2 \mathbf{d_1} - \kappa_1 \mathbf{d_2}$$

Cartesian placement

$$\varphi_{\mathsf{x}} = \boldsymbol{\varphi} \cdot \boldsymbol{e}_{\mathsf{x}}
\varphi_{\mathsf{v}} = \boldsymbol{\varphi} \cdot \boldsymbol{e}_{\mathsf{v}}$$

$$\varphi_{z} = \boldsymbol{\varphi} \cdot \boldsymbol{e}_{z}$$

• First order O.D.E. with nonlinear coefficients



Hard to solve analytically

Numerical simulations.

$$\varphi' = d_3$$

$$d_i' = \kappa imes d_i$$

Deformed placement

$$\varphi_1' - \varphi_2 \kappa_3 + \varphi_3 \kappa_2 = 0
 \varphi_2' - \varphi_3 \kappa_1 + \varphi_1 \kappa_3 = 0
 \varphi_3' - \varphi_1 \kappa_2 + \varphi_2 \kappa_1 = 1$$

Directors

$$\mathbf{d_1}' = \kappa_3 \mathbf{d_2} - \kappa_2 \mathbf{d_3}$$
$$\mathbf{d_2}' = \kappa_1 \mathbf{d_3} - \kappa_3 \mathbf{d_1}$$
$$\mathbf{d_3}' = \kappa_2 \mathbf{d_1} - \kappa_1 \mathbf{d_2}$$

Cartesian placement

$$arphi_{\mathsf{x}} = oldsymbol{arphi} \cdot oldsymbol{e}_{\mathsf{x}} \ arphi_{\mathsf{y}} = oldsymbol{arphi} \cdot oldsymbol{e}_{\mathsf{y}} \ arphi_{\mathsf{z}} = oldsymbol{arphi} \cdot oldsymbol{e}_{\mathsf{z}}$$

First order O.D.E. with nonlinear coefficients



Hard to solve analytically

Numerical simulations.

• For fixed $e, g; \mu$ and ℓ , rod behaviour depends on η .

$$\varphi' = d_3$$

$$d_i' = \kappa imes d_i$$

Deformed placement

$$\begin{aligned}
\varphi_1' - \varphi_2 \kappa_3 + \varphi_3 \kappa_2 &= 0 \\
\varphi_2' - \varphi_3 \kappa_1 + \varphi_1 \kappa_3 &= 0 \\
\varphi_3' - \varphi_1 \kappa_2 + \varphi_2 \kappa_1 &= 1
\end{aligned}$$

Directors

$$egin{aligned} {m{d_1}'} &= \kappa_3 \, {m{d_2}} - \kappa_2 \, {m{d_3}} \ {m{d_2}'} &= \kappa_1 \, {m{d_3}} - \kappa_3 \, {m{d_1}} \ {m{d_3}'} &= \kappa_2 \, {m{d_1}} - \kappa_1 \, {m{d_2}} \end{aligned}$$

Cartesian placement

$$arphi_{\mathsf{X}} = oldsymbol{arphi} \cdot oldsymbol{e}_{\mathsf{X}} \ arphi_{\mathsf{y}} = oldsymbol{arphi} \cdot oldsymbol{e}_{\mathsf{y}} \ arphi_{\mathsf{z}} = oldsymbol{arphi} \cdot oldsymbol{e}_{\mathsf{z}}$$

First order O.D.E. with nonlinear coefficients



Hard to solve analytically

Numerical simulations.

- For fixed $e, g; \mu$ and ℓ , rod behaviour depends on η .
- Detailed study of this control parameter gives a better understanding of the geometry of rod shapes

η analysis:Trivial case

Thick



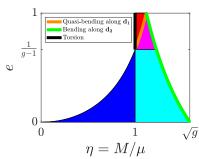




$$\eta=1$$
 Torsion

$$\eta = \sqrt{\mathit{r}_1}$$
 Quasi-bending $\%~ \emph{\textbf{d}}_1$





η analysis:Trivial case

Thin





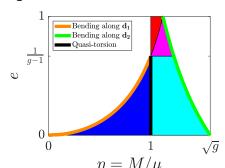
00000000



$$\eta = \sqrt{r_1}$$
 Bending % d_1







η analysis:Trivial case

Thick

= 1

 $\eta=1$ Torsion $\eta = \sqrt{r_1}$ Quasi-bending % $extbf{\emph{d}}_1$

 $\eta = \sqrt{r_2}$ Bending % $\emph{\textbf{d}}_2$

Thin



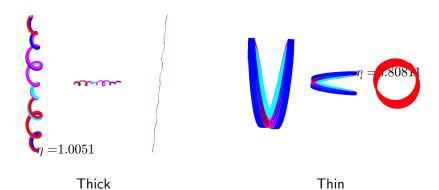


 $\eta = \sqrt{r_1}$ Bending % $m{d_1}$

 $\eta=1$ Quasi-torsion

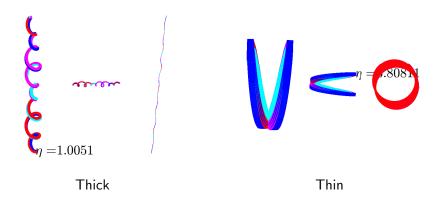
 $\eta = \sqrt{r_2}$ Bending % d_2

What is the behaviour of the rod by varying η for μ fixed???

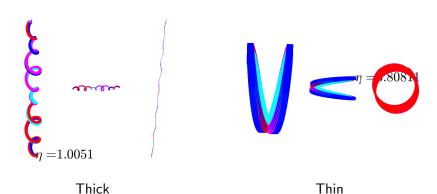


00000000

η analysis: General rod behaviour



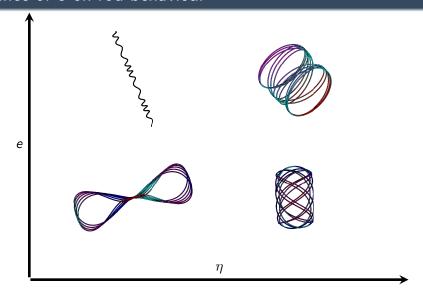
• η dictates shapes of the beam.



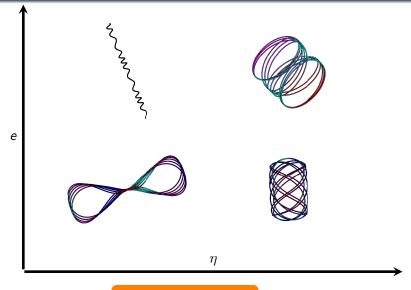
00000000

- η dictates shapes of the beam.
- Increasing η switch rod shape from torsion to bending and vice versa in a continuous way.

Influence of e on rod behaviour



Influence of e on rod behaviour



Pattern size ???

Perspectives



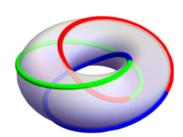
DNA helicoidal shapes



Yarn balls shapes



Chromosome condensation



Torus knots

Conclusion

- Kirshhoff rod subjected to pure moment.
- Two invariants dictate the existence of the solutions.

Conclusion

- Kirshhoff rod subjected to pure moment.
- Two invariants dictate the existence of the solutions.
- Exact solutions in terms of Jacobian elliptic functions.
- Four parameters control the problem:
- Material g.
- External moments η .

- Geometry of the cross section e.
- Internal energy per unit length μ .

Conclusion

- Kirshhoff rod subjected to pure moment.
- Two invariants dictate the existence of the solutions.
- Exact solutions in terms of Jacobian elliptic functions.
- Four parameters control the problem:
- Material g.

• Geometry of the cross section e.

• External moments η .

- Internal energy per unit length μ .
- ullet η controls the shape of the pattern.
- Independent of rod length.

ullet μ controls the size of the pattern.